Titen HD® Heavy-Duty Screw Anchor



IBC





Titen HD Installation Information and Additional Data¹

Characteristic	Symbol	Units				Noi	minal And	hor Diar	neter, d _a	(in.)			
Characteristic	әуніроі	Uints	1	4	3	6		/2	5	%s	3/4		
			Installa	tion Info	rmation								
Drill Bit Diameter	d _{bit}	in.	1/4		3,	8	1	/2	5/8		3/4		
Baseplate Clearance Hole Diameter	d _c	in.	3	/8	3	½	5	/B	3	3/4	7/8		
Maximum Installation Torque	T _{inst,max}	ftlbf	2	4 ²	50) ²	6	5 ²	10	00²		150 ²	
Maximum Impact Wrench Torque Rating	T _{impact,max}	ftlbf	12	25 ³	150 ³		34	10 ³	34	40 ³		385³	
Minimum Hole Depth	h _{hole}	in.	13/4	2%	2¾	31/2	3¾	41/2	41/2	6	41/2	6	63/4
Nominal Embedment Depth	h _{nom}	in.	1%	21/2	21/2	31/4	31/4	4	4	51/2	4	51/2	61/4
Critical Edge Distance	Cac	in.	3	6	211/16	3%	3%6	41/2	41/2	6%	6	6%	75/16
Minimum Edge Distance	C _{min}	in.	1	1/2					1¾				
Minimum Spacing	Smin	in.	1	1/2			3				23/4		3
Minimum Concrete Thickness	h _{min}	in.	31/4	31/2	4	5	5	61/4	6	81/2	6	83/4	10
			Add	ditional l	Data								
Anchor Category	Category	 2:						1					
Yield Strength	f _{ya}	psi	100	,000					97,000				
Tensile Strength	f _{uta}	psi	125	,000					110,000)			
Minimum Tensile and Shear Stress Area	A _{se}	in²	0.0	0.042 0.099		99	0.1	83	0.2	276		0.414	
Axial Stiffness in Service Load Range — Uncracked Concrete	eta_{uncr}	lb./in.	202,000				672,000)					
Axial Stiffness in Service Load Range — Cracked Concrete	β_{cr}	lb./in.	in. 173,000 345,000										

The information presented in this table is to be used in conjunction with the design criteria of ACI 318-19 Chapter 17, ACI 318-14 Chapter 17 and ACI 318-11 Appendix D.

^{2.} T_{inst,max} is the maximum permitted installation torque for the embedment depth range covered by this table using a torque wrench.

^{3.} Timpact,max is the maximum permitted torque rating for impact wrenches for the embedment depth range covered by this table.

Titen HD® Design Information — Concrete

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Titen HD Tension Strength Design Data¹



Characteristic	Symbol	Units				Non	ninal And	hor Dia	meter, d	(in.)			
Characteristic	Symbol	Units		1/4		%	1	/2		%	3/4		
Nominal Embedment Depth	h _{nom}	in.	1%	21/2	21/2	31/4	31/4	4	4	51/2	4	51/2	61/4
Steel Strength in Tens	ion — AC	1 318-1	9 17.6.1	, ACI 318	8-14 17.	4.1 or AC	318-11	Section	n D.5.1				
Tension Resistance of Steel	N _{sa}	lb.	5,	195	10,	890	20,	130	30	,360		45,540	
Strength Reduction Factor — Steel Failure ²	ϕ_{sa}	_	0.65										
Concrete Breakout Strength i	n Tension	6 — AC	318-19	9 17.6.2,	ACI 318	-14 17.4	.2 or ACI	318-11	Section	D.5.2		***	
Effective Embedment Depth	h _{ef}	in.	1.19	1.94	1.77	2.40	2.35	2.99	2.97	4.24	2.94	4.22	4.86
Critical Edge Distance	Cac	in.	3	6	211/16	3%	3%16	41/2	41/2	6%	6	6%	75/16
Effectiveness Factor — Uncracked Concrete	K _{uncr}	-	30				24				27	27 24	
Effectiveness Factor — Cracked Concrete	k _{cr}	4			10			17					
Modification Factor	$\Psi_{c,N}$							1.0					
Strength Reduction Factor — Concrete Breakout Failure ²	ϕ_{cb}	_						0.65					
Pullout Strength in Ten	sion — A	CI 318-	19 17.6.	3, ACI 31	8-14 17	.4.3 or A	CI 318-1	1 Section	n D.5.3				
Pullout Resistance, Uncracked Concrete (f' _c = 2,500 psi)	N _{p,uncr}	lb.	3	3	2,7004	3	3	3	3	9,8104	3	3	3
Pullout Resistance, Cracked Concrete (f' _c = 2,500 psi)	N _{p,cr}	lb.	3	1,9054	1,2354	2,7004	3	3	3,0404	5,5704	3	6,0704	7,1954
Strength Reduction Factor — Pullout Failure ²	ϕ_p	s==						0.65					
Tension Strength for Seismic App	olications	— ACI	318-19	17.10.3,	ACI 318-	-14 17.2	.3.3 or A	CI 318-1	1 Section	on D.3.3.3	3		
Nominal Pullout Strength for Seismic Loads (f' $_{\text{C}} = 2,500 \text{ psi}$)	N _{p,eq}	lb.	_3	1,9054	1,2354	2,7004	3	3	3,0404	5,5704	3,8404	6,0704	7,1954
Strength Reduction Factor — Pullout Failure ²	ϕ_{eq}	8 -3						0.65					

The information presented in this table is to be used in conjunction with the design criteria of ACI 318-19 chapter 17, ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, except as modified below.

^{2.} The strength reduction factor applies when the load combinations from the IBC or ACI 318 are used and the requirements of ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, are met. If the load combinations of ACI 318-11 Appendix C are used, the appropriate strength reduction factor must be determined in accordance with ACI 318-11 D.4.4.

^{3.} Pullout strength is not reported since concrete breakout controls.

^{4.} Adjust the characteristic pullout resistance for other concrete compressive strengths by multiplying the tabular value by (fc, specified / 2,500)05.

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Titen HD® Design Information — Concrete



Titen HD Shear Strength Design Data1



Chamatariatia	Complete	Unit				Nor	ninal And	hor Diar	neter, d _a	(in.)			
Characteristic	Symbol	Unit		/4		%		<u>/2</u>	16	%	3/4		
Nominal Embedment Depth	h _{nom}	in.	1%	21/2	21/2	31/4	31/4	4	4	51/2	4	51/2	61/4
Steel Strength in	Shear (AC	318-1	9 17.7.1	, ACI 318	3-14 17.5	5.1 or AC	318-11	Section	D.6.1)		A)		
Shear Resistance of Steel	V _{sa}	lb.	2,0	020	4,	460	7,4	155	10,	,000	14,950	16,	840
Strength Reduction Factor — Steel Failure ²	ϕ_{sa}	_	- 0.60										
Concrete Breakout Strer	ngth in Sh	ear (AC	318-19	17.7.2	ACI 318-	14 17.5.2	or ACI 3	18-11 Se	ection D.	6.2)			
Outside Diameter	da	in.	0.25 0.375 0.5			0.500 0.625		625	0.750				
Load Bearing Length of Anchor in Shear	ℓ_{e}	in.	1.19	1.94	1.77	2.40	2.35	2.99	2.97	4.24	2.94	4.22	4.86
Strength Reduction Factor — Concrete Breakout Failure ²	ϕ_{cb}	===						0.70					
Concrete Pryout Streng	th in She	ar (ACI	318-19	17.7.3, A	CI 318-1	4 17.5.3	or ACI 31	8-11 Se	ction D.6	.3)			
Coefficient for Pryout Strength	k _{cp}	lb.			1.0					2	2.0		
Strength Reduction Factor — Concrete Pryout Failure ²	ϕ_{cp}	===						0.70					
Steel Strength in Shear for Seisn	nic Applic	ations	(ACI 318	-19 17.1	0.3, ACI	318-14 1	7.2.3.3 o	r ACI 318	3-11 Sec	tion D.3.	3.3)		
Shear Resistance for Seismic Loads	V _{eq}	lb.	1,6	695	2,	355	4,7	790	8,0	000		9,350	
Strength Reduction Factor — Steel Failure ²	ϕ_{eq}	=					3-	0.60			19:		

- 1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318-19 Chapter 17, ACI 318-14 Chapter 17 and ACI 318-11 Appendix D, except as modified below.
- 2. The strength reduction factor applies when the load combinations from the IBC or ACI 318 are used and the requirements of ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, are met. If the load combinations of ACI 318-11 Appendix C are used, the appropriate strength reduction factor must be determined in accordance with ACI 318-11 D.4.4.

Titen HD Tension and Shear Strength Design Data for the Soffit of Normal-Weight or Sand-Lightweight Concrete over Steel Deck^{1,6,7}



			Nominal Anchor Diameter, d _a (in.)											
0		Units	Lower Flute							Upper Flute				
Characteristic	Symbol		Figu	ıre 2		Figu	ıre 1		Figu	ıre 2	Figu	ıre 1		
			1/4		3⁄8		1/2		1/4		3%	1/2		
Nominal Embedment Depth	h _{nom}	in.	1%	21/2	1%	21/2	2	31/2	1%	21/2	1%	2		
Effective Embedment Depth	h _{ef}	in.	1.19	1.94	1.23	1.77	1.29	2.56	1.19	1.94	1.23	1.29		
Pullout Resistance, concrete on steel deck (cracked)2,3,4	N _{p,deck,cr}	lb.	420	535	375	870	905	2,040	655	1,195	500	1,700		
Pullout Resistance, concrete on steel deck (uncracked) ^{2,3,4}	N _{p,deck,uncr}	lb.	995	1,275	825	1,905	1,295	2,910	1,555	2,850	1,095	2,430		
Steel Strength in Shear, concrete on steel deck ⁵	V _{sa, deck}	lb.	1,335	1,745	2,240	2,395	2,435	4,430	2,010	2,420	4,180	7,145		
Steel Strength in Shear, Seismic	V _{sa, deck,eq}	lb.	870	1,135	1,434	1,533	1,565	2,846	1,305	1,575	2,676	4,591		

- The information presented in this table is to be used in conjunction with the design criteria of ACI 318-19 Chapter 17, ACI 318-14 Chapter 17 and ACI 318-11 Appendix D, except as modified below.
- Concrete compressive strength shall be 3,000 psi minimum. The characteristic pullout resistance for greater compressive strengths shall be increased by multiplying the tabular value by (f^{*}_{C.specified} /3,000)^{0.5}.
- 3. For anchors installed in the soffit of sand-lightweight or normal-weight concrete over steel deck floor and roof assemblies, as shown in Figure 1 and Figure 2, calculation of the concrete breakout strength may be omitted.
- 4. In accordance with ACI 318-19 Section 17.6.3.2.1, ACI 318-14 Section 17.4.3.2 or ACI 318-11 Section D.5.3.2, the nominal pullout strength in cracked concrete for anchors installed in the soffit of sand-lightweight or normal-weight concrete over steel deck floor and roof assemblies N_{p,deck,cr} shall be substituted for N_{p,cr}. Where analysis indicates no cracking at service loads, the normal pullout strength in uncracked concrete N_{p,deck,uncr} shall be substituted for N_{p,uncr}.
- 5. In accordance with ACI 318-19 Section 17.7.1.2(c), ACI 318-14 Section 17.5.1.2(c) or ACI 318-11 Section D.6.1.2(c), the shear strength for anchors installed in the soffit of sand-lightweight or normal-weight concrete over steel deck floor and roof assemblies V_{sa,deck,eq} shall be substituted for V_{sa}.
- 6. Minimum edge distance to edge of panel is $2h_{\it eff}$
- 7. The minimum anchor spacing along the flute must be the greater of 3h_{oft} or 1.5 times the flute width.

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Titen HD[®] Design Information — Concrete



Titen HD Anchor Tension and Shear Strength Design Data in the Topside of Normal-Weight Concrete or Sand-Lightweight Concrete over Steel Deck^{1,2,3,4}

IBC	1	=	
	2812 2862	280 280	1000000

			Nominal Anchor Diameter, d _a (in.) Figure 3								
Design Information	Symbol	Units									
			1/4	%		/2					
Nominal Embedment Depth	h _{nom}	in.	1%	21/2	31/4	4					
Effective Embedment Depth	h _{ef}	in.	1.19	1.77	2.35	2.99					
Minimum Concrete Thickness ⁵	h _{min,deck}	in.	21/2	31/4	41/2	41/2					
Critical Edge Distance	C _{ac,deck,top}	in.	3¾	71/4	9	9					
Minimum Edge Distance	C _{min,deck,top}	in.	31/2	3	21/2	21/2					
Minimum Spacing	S _{min,deck,top}	in.	31/2	3	3	3					

- 1. For anchors installed in the topside of concrete-filled deck assemblies, as shown in Figure 3, the nominal concrete breakout strength of a single anchor or group of anchors in shear, V_{cb} or V_{cbg}, respectively, must be calculated in accordance with ACI 318-19 Section 17.7.2, ACI 318-14 Section 17.5.2 or ACI 318-11 Section D.6.2, using the actual member thickness, h_{min,deck}, in the determination of A_{vc}.
- 2. Design capacity shall be based on calculations according to values in the tables featured on pp. 69 and 70.
- 3. Minimum flute depth (distance from top of flute to bottom of flute) is 1 1/2" (see Figure 3).
- 4. Steel deck thickness shall be minimum 20 gauge.
- 5. Minimum concrete thickness (hmin,deck) refers to concrete thickness above upper flute (see Figure 3).

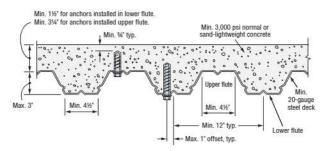


Figure 1. Installation of %"- and ½"-Diameter Anchors in the Soffit of Concrete over Steel Deck

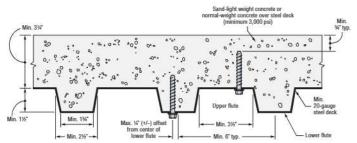


Figure 2. Installation of ¼"-Diameter Anchors in the Soffit of Concrete over Steel Deck

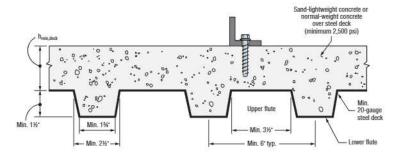


Figure 3. Installation of ¼"- and %"-Diameter Anchors in the Topside of Concrete over Steel Deck

Titen HD[®] Design Information — Masonry



Titen HD Allowable Tension and Shear Loads in 8" Lightweight, Medium-Weight and Normal-Weight Grout-Filled CMU

IBC	1	-	*
IDU	357 352	35 35	1

		Minimum	Critical Edge	Minimum Edge	Critical	Values for 8" Lightweight, Medium-Weight or Normal-Weight Grout-Filled CMU					
Size in.	Diameter	Embedment Depth	Distance C _{crit}	Distance C _{min}	Spacing Distance	Tension Load		Shear Load			
(mm)	in.	in. (mm)	in. (mm)	in. (mm)	in (mm)	Ultimate lb. (kN)	Allowable lb. (kN)	Ultimate lb. (kN)	Allowable lb. (kN)		
	la.		Ancho	or Installed in t	he Face of the	CMU Wall (See Fig	jure 4)		N.		
1/4 (6.4)	1/4	2½ (64)	4 (102)	1 1/4 (32)	4 (102)	2,050 (9.1)	410 (1.8)	2,500 (11.1)	500 (2.2)		
3 /8 (9.5)	3/8	2¾ (70)	12 (305)	4 (102)	6 (152)	2,390 (10.6)	480 (2.1)	4,340 (19.3)	870 (3.9)		
½ (12.7)	1/2	3½ (89)	12 (305)	4 (102)	8 (203)	3,440 (15.3)	690 (3.1)	6,920 (30.8)	1,385 (6.2)		
5% (15.9)	%	4½ (114)	12 (305)	4 (102)	10 (254)	5,300 (23.6)	1,060 (4.7)	10,420 (46.4)	2,085 (9.3)		
3/4 (19.1)	3/4	5½ (140)	12 (305)	4 (102)	12 (305)	7,990 (35.5)	1,600 (7.1)	15,000 (66.7)	3,000 (13.3)		

- 1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.
- 2. Values for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.
- 3. The masonry units must be fully grouted.

Mechanical Anchors

- 4. The minimum specified compressive strength of masonry, f'm, at 28 days is 1,500 psi.
- 5. Embedment depth is measured from the outside face of the concrete masonry unit.
- 6. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied loads.
- 7. Refer to allowable load-adjustment factors for spacing and edge distance on pp. 78-79.

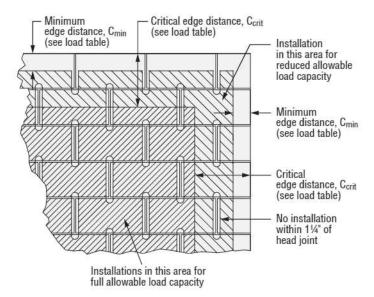


Figure 4. Shaded Area = Placement for Full and Reduced Allowable Load Capacity in Grout-Filled CMU

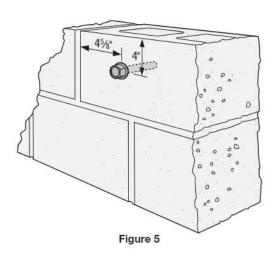


Titen HD Allowable Tension and Shear Loads in 8" Lightweight, Medium-Weight and Normal-Weight Hollow CMU

			+
IRC	1	-	-
	255 353	5895 5895	

		Embedment	Minimum		8" Hollow CMU on CMU		
Size in.	Drill Bit Diameter in.	Depth ⁴ in.	Edge Distance in.	Tensio	n Load	Shea	r Load
(mm)	III.	(mm)	(mm)	Ultimate lb. (kN)	Allowable lb. (kN)	Ultimate lb. (kN)	Allowable lb. (kN)
		Anch	nor Installed in Fac	e Shell (See Figu	re 5)		200
% (9.5)	3/8	13/4 (45)	4 (102)	720 (3.2)	145 (0.6)	1,240 (5.5)	250 (1.1)
½ (12.7)	1/2	13/4 (45)	4 (102)	760 (3.4)	150 (0.7)	1,240 (5.5)	250 (1.1)
% (15.9)	5%8	13/4 (45)	4 (102)	800 (3.6)	160 (0.7)	1,240 (5.5)	250 (1.1)
3/4 (19.1)	3/4	13/4 (45)	4 (102)	880 (3.9)	175 (0.8)	1,240 (5.5)	250 (1.1)

- The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC. Note: No installation within 4%" of bed joint of hollow masonry block wall.
- 2. Values for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.
- 3. The minimum specified compressive strength of masonry, f'_m, at 28 days is 1,500 psi.
- Embedment depth is measured from the outside face of the concrete masonry unit and is based on the anchor being embedded an additional ½"- through 1 ¼"-thick face shell.
- 5. Allowable loads may not be increased for short-term loading due to wind or seismic forces.
- 6. CMU wall design must satisfy applicable design standards and be capable of withstanding applied loads.
- 7. Do not use impact wrenches to install in hollow CMU.
- 8. Set drill to rotation-only mode when drilling into hollow CMU.
- 9. The tabulated allowable loads are based on one anchor installed in a single cell.
- 10. Distance from centerline of anchor to head joint shall be a minimum of 4%".



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Titen HD® Design Information — Masonry



Titen HD Allowable Tension and Shear Loads in

8" Lightweight, Medium-Weight and Normal-Weight Grout-Filled CMU Stemwall



		Embed.	Minimum		Critical	8" Grout-Filled CMU Allowable Loads Based on CMU Strength, $f_{\it m}=1,\!500$ psi										
Size in.	Drill Bit Diameter	Depth	Edge Distance	End Distance	Spacing Distance	Ter	nsion	Shear Perpen	dicular to Edge	Shear Para	allel to Edge					
(mm)	in.	in. (mm)	in. (mm)	in. (mm)	in. (mm)	Ultimate lb. (kN)	Allowable lb. (kN)	Ultimate lb. (kN)	Allowable lb. (kN)	Ultimate lb. (kN)	Allowable lb. (kN)					
				Anchor Ir	stalled in C	ell Opening or \	Web (Top of Wall) (See Figure 6)								
½ (12.7)	1/2	4 ½ (114)	13/4 (45)	8 (203)	8 (203)	2,860 (12.7)	570 (2.5)	800 (3.6)	160 (0.7)	2,920 (13.0)	585 (2.6)					
% (15.9)	5/8	4½ (114)	13/4 (45)	10 (254)	10 (254)	2,860 (12.7)	570 (2.5)	800 (3.6)	160 (0.7)	3,380 (15.0)	675 (3.0)					

- 1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.
- 2. Values are for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.
- 3. The masonry units must be fully grouted.
- 4. The minimum specified compressive strength of masonry, f'm, at 28 days is 1,500 psi.
- 5. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied design loads.
- 6. Loads are based on anchor installed in either the web or grout-filled cell opening in the top of wall.

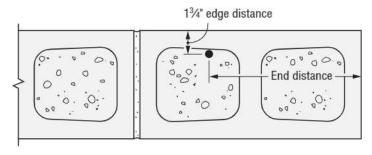


Figure 6.
Anchor Installed in Top of Wall at 13/4" Edge Distance

Titen HD Allowable Tension and Shear Loads in 8" Medium-Weight and Normal-Weight Grout-Filled CMU Stemwall



		Embed.	Minimum	Minimum	Critical	8" Gro	ut-Filled CMU A	llowable Loads I	Based on CMU St	rength, f'm = 2	,000 psi			
Size in.	Drill Bit Diameter	Depth in.	Edge Distance	End Distance		End Distance		Spacing Distance	Ten	sion	Shear Perpendicular to Edge		Shear Parallel to Edge	
(mm)	in.	(mm)	in. (mm)	in. (mm)	in. (mm)	Ultimate lb. (kN)	Allowable lb. (kN)	Ultimate lb. (kN)	Allowabłe lb. (kN)	Ultimate lb. (kN)	Allowable lb. (kN)			
	2- 0			Anch	or Installed	n Cell Opening	(Top of Wall) (Se	ee Figure 7)			90 			
½ (12.7)	1/2	41/2	3	12	12	5,800	1,160	2,750	550	7,500	1,500			
% (15.9)	5/8	(114) (76)			(25.8)	(5.2)	(12.2)	(2.5)	(33.4)	(6.7)				

- 1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.
- Values are for 8"-wide, medium-weight and normal-weight concrete masonry units.
- 3. The masonry units must be fully grouted.
- 4. The minimum specified compressive strength of masonry, f'_{m_h} at 28 days is 2,000 psi.
- 5. Allowable loads are not permitted to be increased for short-term loading due to wind or seismic forces.
- 6. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied design loads.
- 7. Loads are based on anchor installed in grout-filled cell opening in the top of wall.

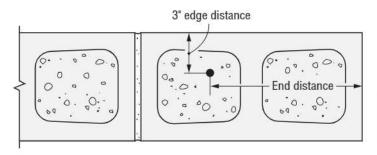


Figure 7.
Anchor Installed in Top of Wall at 3" Edge Distance

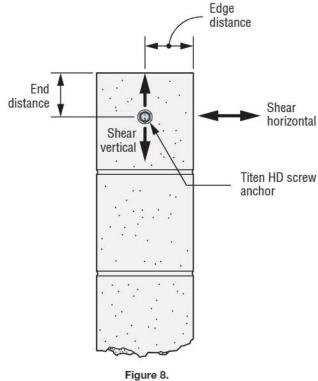


Titen HD Allowable Tension and Shear Loads in End of 8" Lightweight, Medium-Weight and Normal-Weight Grout-Filled CMU Wall

IBC	1	→	*
	200 200	XV 552	50000 000

	o- Drill Bit		Minimum	Minimum	Minimum	Allowable Loads			
Size (in.)	Drill Bit Diameter (in.)	Embedment Depth (in.)	Edge Distance (in.)	End Distance (in.)	Spacing (in.)	Tension (lbf)	Shear Vertical (lbf)	Shear Horizontal (lbf)	
1/4	1/4	2%	313/16	1¾	4	310	215	375	
3/8	3/8	2%	313/16	1¾	6	335	215	375	

- 1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.
- 2. Values are for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.
- 3. The masonry units must be fully grouted.
- 4. The minimum specified compressive strength of masonry, f'm, at 28 days is 2,000 psi.
- 5. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied design loads.
- 6. Minimum edge and end distances are measured from anchor centerline to the edge and end of the CMU masonry wall, respectively. Refer to Figure 8 below.



Anchor Installed in End of Grout-Filled CMU Wall

Titen HD[®] Design Information — Masonry



Titen HD Allowable Tension and Shear Loads in End of 8" Lightweight, Medium-Weight and Normal-Weight Hollow CMU Wall

			(REIN)
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	o: Drill Bit	Drill Bit Embedment		Minimum Minimum		Allowable Loads		
Size (in.)	Diameter (in.)	Depth (in.)	Edge Distance (in.)	End Distance (in.)	Minimum Spacing (in.)	Tension (lbf)	Shear Vertical (lbf)	Shear Horizontal (lbf)
1/4	1/4	2%	313/16	13/4	4	130	105	120
3/8	3/8	2%	313/16	13/4	6	130	115	125

- 1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC,
- 2. Values for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.
- 3. The minimum specified compressive strength of masonry, f'_m, at 28 days is 2,000 psi.
- Embedment depth is measured from the outside face of the concrete masonry unit and is based on the anchor being embedded an additional 1 %"- through 1 %"-thick face shell.
- 5. Allowable loads may not be increased for short-term loading due to wind or seismic forces.
- 6. CMU wall design must satisfy applicable design standards and be capable of withstanding applied loads.
- 7. Do not use impact wrenches to install in hollow CMU.
- 8. Set drill to rotation-only mode when drilling into hollow CMU.
- Minimum edge and end distances are measured from anchor centerline to the edge and end of the CMU masonry wall, respectively. Refer to Figure 9 below.
- 10. Anchors must be installed a minimum of 1 1/2" from centerlie of bed joints. See Figure 9 for prohibited anchor installation locations.

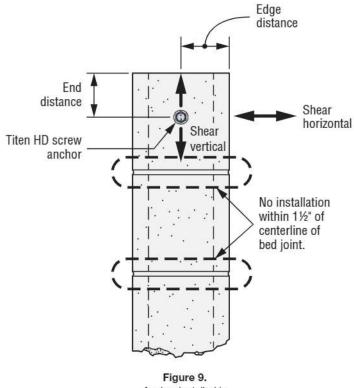


Figure 9.
Anchor Installed in
End of Hollow CMU Wall

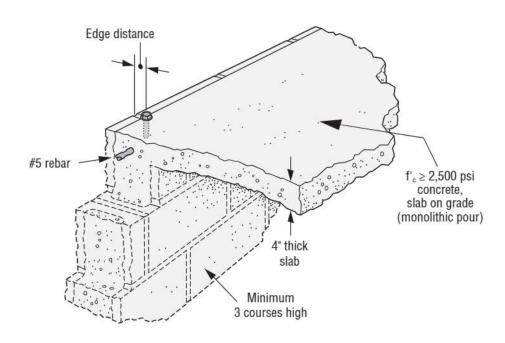


Titen HD Allowable Tension Loads for 8" Lightweight, Medium-Weight and Normal-Weight CMU Chair Blocks Filled with Normal-Weight Concrete

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Size	Drill Bit	Minimum Embedment	Minimum Edge	Critical		d CMU Chair Block s Based on CMU Strength
in. Diameter (mm) (in.)	Diameter	Depth in. (mm)	Distance in. (mm)	Spacing in. (mm)	Ultimate lb. (kN)	Allowable lb. (kN)
		23/s (60)	13/4 (44)	9½ (241)	3,175 (14.1)	635 (2.8)
3/8 (9.5)		3% (86)	13/4 (44)	13½ (343)	5,175 (23.0)	1,035 (4.6)
		5 (127)	2 1/4 (57)	20 (508)	10,584 (47.1)	2,115 (9.4)
1/2		8 (203)	2 1/4 (57)	32 (813)	13,722 (61.0)	2,754 (12.2)
(12.7) 1/2	10 (254)	2 1/4 (57)	40 (1016)	16,630 (74.0)	3,325 (14.8)	
5/8 (15.9)	5/8	5½ (140)	13/4 (44)	22 (559)	9,025 (40.1)	1,805 (8.1)

^{1.} The tabulated allowable loads are based on a safety factor of 5.0.



Values are for 8"-wide concrete masonry units (CMU) filled with concrete, with minimum compressive strength of 2,500 psi and poured monolithically with the floor slab.

^{3.} Center #5 rebar in CMU cell and concrete slab as shown in the illustration below.



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Load-Adjustment Factors for Titen HD Anchors in Face-of-Wall Installation in 8" Grout-Filled CMU: Edge Distance and Spacing, Tension and Shear Loads

How to use these charts:

- The following tables are for reduced edge distance and spacing.
- Locate the anchor size to be used for either a tension and/or shear load application.
- Locate the embedment (E) at which the anchor is to be installed.
- Locate the edge distance (c_{act}) or spacing (s_{act}) at which the anchor is to be installed.
- The load adjustment factor (f_c or f_s) is the intersection of the row and column.
- Multiply the allowable load by the applicable load adjustment factor.
- Reduction factors for multiple edges or spacings are multiplied together.

Edge	Distance	Tension	(f_)

	Dia.	1/4	3/8	1/2	5/8	3/4
	E	21/2	23/4	31/2	41/2	51/2
c _{act} (in.)	C _{Cr}	4	12	12	12	12
(111.)	c _{min}	1.25	4	4	4	4
	f _{cmin}	0.77	1.00	1.00	0.83	0.66
1.25		0.77				
2		0.83				
3		0.92				
4		1.00	1.00	1.00	0.83	0.66
6		1.00	1.00	1.00	0.87	0.75
8		1.00	1.00	1.00	0.92	0.83
10		1.00	1.00	1.00	0.96	0.92
12		1.00	1.00	1.00	1.00	1.00

See footnotes below.

Edge Distance Shear (f_c) Shear Load Parallel to Edge or End

	Dia.	1/4	3/8	1/2	5/8	3/4
8	E	21/2	23/4	31/2	41/2	51/2
c _{act} (in.)	Ccr	4	12	12	12	12
(111.)	c _{min}	1.25	4	4	4	4 0.44
	f _{cmin}	0.58	0.77	0.48	0.46	
1.25		0.58				
2		0.69				
3		0.85				
4		1.00	0.77	0.48	0.46	0.44
6		1.00	0.83	0.61	0.60	0.58
8		1.00	0.89	0.74	0.73	0.72
10		1.00	0.94	0.87	0.87	0.86
12		1.00	1.00	1.00	1.00	1.00

See footnotes below.

Edge Distance Shear (f_c) Shear Load Perpendicular to Edge or End (Directed Towards Edge or End)

	Dia.	1/4	3/8	1/2	5/8	3/4
Ì	E	21/2	23/4	3 1/2	4 1/2	5 1/2
c _{act} (in.)	C _C r	4	12	12	12	12
(111.)	C _{min}	1.25	4	4	4	4
	f _{cmin}	0.71	0.58	0.38	0.30	0.21
1.25		0.71				
2		0.79				
3		0.89				
4		1.00	0.58	0.38	0.30	0.21
6		1.00	0.69	0.54	0.48	0.41
8		1.00	0.79	0.69	0.65	0.61
10		1.00	0.90	0.85	0.83	0.80
12		1.00	1.00	1.00	1.00	1.00

^{1.} E = embedment depth (inches).

^{2.} cact = actual end or edge distance at which anchor is installed (inches).

^{3.} c_{cr} = critical end or edge distance for 100% load (inches).

^{4.} c_{min} = minimum end or edge distance for reduced load (inches).

^{5.} f_c = adjustment factor for allowable load at actual end or edge distance.

^{6.} f_{ccr} = adjustment factor for allowable load at critical end or edge distance. f_{ccr} is always = 1.00.

 $^{7.}f_{\mbox{\scriptsize cmin}}$ = adjustment factor for allowable load at minimum end or edge distance.

^{8.} $f_c = f_{cmin} + [(1 - f_{cmin}) (c_{act} - c_{min}) / (c_{cr} - c_{min})].$



Load-Adjustment Factors for Titen HD Anchors in Face-of-Wall Installation in 8" Grout-Filled CMU: Edge Distance and Spacing, Tension and Shear Loads (cont.)

How to use these charts:

- The following tables are for reduced edge distance and spacing.
- Locate the anchor size to be used for either a tension and/or shear load application.
- 3. Locate the embedment (E) at which the anchor is to be installed.
- Locate the edge distance (c_{act}) or spacing (s_{act}) at which the anchor is to be installed.
- The load adjustment factor (f_c or f_s) is the intersection of the row and column.
- Multiply the allowable load by the applicable load adjustment factor.
- Reduction factors for multiple edges or spacings are multiplied together.

Edge Distance Shear (f _c)
Shear Load Perpendicular to Edge or End
(Directed Away from Edge or End)

	Dia.	1/4	3/8	1/2	5/8	3/4
	E	21/2	23/4	31/2	4 1/2	5 1/2
c _{act} (in.)	C _C r	4	12	12	12	12
(111.)	c _{min}	1.25	4	4	4	4
	f _{cmin}	0.71	0.89	0.79	0.58	0.38
1.25		0.71				
2		0.79				
3		0.89				
4		1.00	0.89	0.79	0.58	0.38
6		1.00	0.92	0.84	0.69	0.54

0.95

0.97

1.00

1.00

1.00

1.00

0.90

0.95

1.00

0.79

0.90

1.00

0.69

0.85

1.00

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Spacing Tension (f_s)

8

10

12

s _{act} (in.)	Dia.	1/4	3/8	1/2	5/8	3/4							
	E S _{Cr} S _{min} f _{smin}	2½ 4 2 0.66	2 ³ / ₄ 6 3 0.87	3 ½ 8 4 0.69	4 ½ 10 5 0.59	5½ 12 6 0.50							
							2		0.66				
							3		0.83	0.87			
							4		1.00	0.91	0.69		
5		Î	0.96	0.77	0.59								
6			1.00	0.85	0.67	0.50							
8				1.00	0.84	0.67							
10					1.00	0.83							
12						1.00							

Spacing Shear (f_s)

s _{act} (in.)	Dia.	1/4	3/8	1/2	5/8	3/4							
	E S _{Cr} S _{min} f _{smin}	2½ 4 2 0.87	2¾ 6 3 0.62	3 ½ 8 4 0.62	4 ½ 10 5 0.62	5 ½ 12 6 0.62							
							2		0.87				
							3		0.93	0.62			(c)
							4		1.00	0.75	0.62		
5			0.87	0.72	0.62								
6			1.00	0.81	0.70	0.62							
8				1.00	0.85	0.75							
10				,	1.00	0.87							
12						1.00							

^{1.} E = embedment depth (inches).

^{2.} sact = actual spacing distance at which anchors are installed (inches).

 $^{3.} s_{cr}$ = critical spacing distance for 100% load (inches).

^{4.} s_{min} = minimum spacing distance for reduced load (inches).

^{5.} f_s = adjustment factor for allowable load at actual spacing distance.

^{6.} f_{scr} = adjustment factor for allowable load at critical spacing distance. f_{scr} is always = 1.00.

^{7.} f_{smin} = adjustment factor for allowable load at minimum spacing distance.

^{8.} $f_s = f_{smin} + [(1 - f_{smin}) (s_{act} - s_{min}) / (s_{cr} - s_{min})].$